



Task C Rapid and low-cost in-situ building vulnerability assessment

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On-site campaign in Thessaloniki, Greece

AUTH, September-October 2015

Building of the Faculty of Philosophy







5-story RC frame with masonry walls and elevator shaft

Dynamik

Statik

8-story RC frame with a RC shaft

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On-site campaign in Cologne, Germany

(November-December 2015)

NN	School	General information
1	Humboldt-Gymnasium	Year of construction - 1956 Number of schoolchildren - 1200 Structural system – mixed, RC, masonry
2	Alfred-Müller-Armack Berufskolleg	Year of construction - 2007 Number of pupils – 3000 (800) Structural system – masonry shear walls
3	Henry-Ford-Realschule	Year of construction – ca. 1965 Number of schoolchildren - 850 Structural system – mixed, RC, masonry
4	Berufskolleg Ehrenfeld	Year of construction – ca. 1960 Number of schoolchildren - not specified Structural system – mixed, RC, masonry
5	Otto-Lilienthal-Schule	Year of construction - 1969 Number of schoolchildren – not specified Structural system – mixed, RC, masonry
6	Gymnasium Thusnelda-straße	Year of construction - 1960s Number of schoolchildren - 843 Structural system – mixed, RC, masonry
7	Gymnasium Kreuzgasse	Year of construction – not specified Number of schoolchildren - 979 Structural system – mixed, RC, masonry



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School buildings, Cologne, Germany



RC frame, masonry in-fill and shear walls



Mixed: RC, masonry



Masonry shear walls

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School buildings, Cologne, Germany



Mixed: RC, masonry



Mixed: RC, masonry



shear wall, masonry







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On-site campaign in L'Aquila, Italy

(May-June 2016)



Partly damaged building since 2009 RC frame, in-fill walls



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Data needed for the modelling and vulnerability assessment

No.	Data type	Ranking
1	Lateral load-resisting system and material of bearing structures	1
2	Overall dimensions and shape of the building	1
3	Presence and location of separation lines	1
4	Presence of irregularities (physical or geometrical / in plan or in elevation)	1
5	Dimensions and location of structural elements (columns, walls, slabs)	1
6	Cross-sections of the structural members and their material properties (strength, elastic moduli, specific density)	1
7	Year of construction (modification)	2
8	Occupancy of the building	2
9	Non-structural elements and other building components, which can contribute to the stiffness and/or mass distribution	2
10	State of the preservation of the building (structural system)	2
11	Depth and type of foundation	2
12	Local soil conditions	2
13	Position of the building with respect to the neighboring buildings	2
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Typical structural members



Structural members

- Columns
- Girders
- Walls
- Slabs
- Shafts (lift, stairs)

with their

- Position
- Dimensions
- Material properties







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Data collection tools



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Ambient vibration measurements







Data input in MS Excel

1st sheet = user interface

	A	В	С	D	E	F	G	Н	I	J	К	L	M
1	Building												
2	Number of Stories		Length X [m]		E-Modulus [MN i m²]		Poisson's ratio		E-Modulus, Walls [MN/m²]	Meas./Calc. Frequency, X [Hz]			0
3	Total Height		Length Y [m]		Siffness Ratio		Number of Column Types			Meas./Calc. Frequency, Y [Hz]			0
4	Uniform Gri	id of Column	IS										
6	Current Story	Total number of columns in X- direction	Total number of columns in Y- direction	Bay Width, X [m]	Bay Width, Y [m]	Cross-section Width, X [m]	Cross-section Width, Y [m]	Diameter [m] (if round cross- section, else 0)	First Column Coordinate, X [m]	First Column Coordinate, Y [m]	Material Density [kN/m3]	Einfüç	gen
7													
8	Non-unifor	m Grid of Co	lums										
10	Current Story	Column Coordinate, X [m]	Column Coordinate, Y [m]	Cross-section Width, X [m]	Cross-section Width, Y [m]	Diameter [m] (if round cross- section, else 0)	Material Density [kN I m3]	Einf	ügen				
11													
12 13	Hollow Shat	ft											
14	Current Story	X-coordinate (center) [m]	Y-coordinate (center) [m]	Cross-section Width, X [m]	Cross-section Width, Y [m]	Wall Thickness, X [m]	Wall Thickness, Y [m]	Material Density [kNIr	Einfüger	1			
15									u				
16 17	Walls							,		1			
18	Current Story	First Point Coordinate in Plan, X1[m]	First Point Coordinate in Plan, Y1[m]	End Point Coordinate in Plan, X1[m]	End Point Coordinate in Plan, Y1[m]	Wall Thickness, [m]	Material Density [kNłm3]	Eint	fügen				
19	N												
20	Number of	Elements											
21	Story	Number of columns	Number of shafts	Number of walls									



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Linear model check and tuning in MS Excel



20	Number of E	Elements		
21	Story	Number of columns	Number of shafts	Number of walls
22	1	32	1	2
23	2	32	1	8
24	3	32	1	8
25	4	32	1	8
26	5	32	1	8
32		Stiffnesses [MN/m]	
33		Story	Кх	Ку
34		1	1224.574208	1867.018663
35		2	7560.66339	4861.827031
36		3	7560.66339	4861.827031
37		4	7560.66339	4861.827031
38		5	9420.398476	6396.379231
44		Calculated F	requencies [Hz]
45		Mode	x	Y
46		1	1.601816004	1.767334209
47		2	6.634602475	5.789989168
48		3	12.08460813	9.984985082
49		4	16.45251741	13.38894828
50		5	18.87130599	15.29247381
56				
57				
58	Column cros	s-sections		
59	Current No.	X-Width [m]	Y-Width [m]	Radius [m]

Frequency difference < 20% => OK, green Frequency difference > 20% => modify, red

uncertainties

lasses and	Weights				
Story	Story Mass [kN/m]	Slab density [kN/m ³]	Story Height [m]	Slab Thickness [m]	Weight [kN]
1	364.95	50	5.8	0.4	19961.415
2	783	50	5.1	0.4	20816.1
3	783	50	5.1	0.4	20816.1
4	783	50	5.1	0.4	20385.45
5	783	50	4	0.4	18545.4

Concrete Properties

Earthquake Forces

lambda	1	Spectral acceleration	2.45
Weight [kN]	100524.465	Shear Force Fges [MN]	246.2849393

58	Column cros	ss-sections				Shaft cross-	sections					Strength [MPa]	30
59	Current No.	X-Width [m]	Y-Width [m]	Radius [m]	Concrete cover [m]	Current No.	X-Width [m]	Y-Width [m]	Wall Thickness, X [m]	Wall Thickness, Y [m]		Reinforcement ratio	0.015
60	1	0.5	0.75	0	0.03	1	1.7	3.9	0.15	0.15		Tens. strength [MPa]	2.2
	Dyna	mik										CIR	
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Earthquake loading and capacity





Force-based limit state assessment

Considered limit states:

LS1: cracks (IO = immediate occupancy) LS2: yielding (DL = damage limitation)

LS3: collapse (NC = near collapse)

Limit states are determined individually for each story by comparing the EQ shear force with corresponding limit state force.



	А	В	С	D	E	F	G	Н	I.	J	К		
57	Stiffnesses f	or LS2 (DL) [I	MN/m]		Stiffnesses f	or LS3 (NC) [MN/m]		Earthquake Forces [MN]				
58	Story	Kx	Ку		Story	Кх	Ку		Story	Force x	Force y		
59	1	372.4023786	827.4240258		1	82.26828406	273.7881038		1	40.65073695	32.41949512		
70	2	4797.979814	2957.408525		2	3245.500734	1785.880412		2	48.11448874	45.06529491		
71	3	4755.07782	2878.319765		3	3224.925779	1719.394627		3	52.54018378	54.0222851		
72	4	4708.255894	2788.379897		4	3208.667273	1667.673076		4	54.40175504	58.98240049		
73	5	5959.617656	3761.467554		5	4126.531119	2399.274547		5	50.57777474	55.79546364		
79				-									

235 Limit state assessment

236 X-direction

237	Story	EQ Force [MN]	LS1 Force [MN]	LS2 Force [MN]	LS3 Force [MN]
238	1	246.2849393	4.746546254	20.78882127	22.87858781
239	2	195.7071645	31.89646838	301.1644601	1102.459691
240	3	141.3054095	27.6340501	290.0644197	1334.017535
241	4	88.76522569	23.37163172	279.5643801	1652.873636
242	5	40.65073695	30.49733346	441.6718571	3431.459118



	Y-direction				
	Story	EQ Force [MN]	LS1 Force [MN]	LS2 Force [MN]	LS3 Force [MN]
	1	246.2849393	6.131266703	33.37581799	38.69791476
	2	190.4894756	17.95616911	133.1247826	312.3054249
	3	131.5070751	15.019346	123.3464814	384.473826
	4	77.48479003	12.08252286	113.6312262	500.433487
l	5	32.41949512	15.44320505	186.2155253	1274.251465





Simplified integral structural model (SISM)



Stiffness matrix	$\mathbf{K} = \begin{bmatrix} k_1 + k_2 & -k_2 & 0\\ -k_2 & k_2 + k_3 & -k_3\\ 0 & -k_3 & k_3 \end{bmatrix}$
Mass matrix	$\boldsymbol{M} = \begin{bmatrix} m_1 & 0 & 0 \\ 0 & m_2 & 0 \\ 0 & 0 & m_3 \end{bmatrix}$
Modal analysis:	$(K-\Omega M)\Phi=0$

Linear model check and improvement is done by comparison of measured and calculated eigenfrequencies and mode shapes.

Each story will be replaced by an equivalent beam beam element with stiffness k_i and mass m_i . Both bending and shear deformations are taken into account.





Linear SISM: Frame part





Linear SISM: shear walls and shafts





Qw

in-fill

wall

В

walls (shear)

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Shaft:

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Considered limit states:

Nonlinear SISM

LS1: cracks (IO = immediate occupancy) LS2: yielding (DL = damage limitation) LS3: collapse (NC = near collapse)

Moment-curvature Force-displacement relationship relationship 0,8 2 M, [KNm] 1,5 F, [MN] 0,6 Frame 1 0,4 Shaft: 0,5 0,2 0 0 0,05 0,005 0 0 0,01 Mechanical model к u, [m] $K_{W} = \begin{cases} 1.0 K_{W} \text{ for LS1} \\ 0.7 K_{W} \text{ for LS2} \\ 0.5 K_{W} \text{ for LS3} \end{cases}$ Wall: Empirical model Dynamik Statik 18/37

Case study: The building of the Faculty of Philosophy, AUTH







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Original design drawings





Faculty building AUTH





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In-situ structural survey and data collection







Main structural members of the central unit







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Summary: main structural elements



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Spatial arrangement of sensors





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1st mode (f=1.60 Hz)







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SIB

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2nd mode (f=1.72 Hz)







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3rd mode (f=1.76)

1





Model check by use of vibration monitoring

	А	В	С	D	E	F	G	Н	1	J	К	L	М
	Building												
1	Dunung												
	Number of				E Modulus				E Modulus	Meas./Calc.			
	Number of	5	Length X [m]	33.6	E-IVIODUIUS	33000	Poisson's ratio	0.2	E-IVIOUUIUS,	Frequency, X	1.72		1.601816004
	stones				[IVIIN/TTI]				wans [wiw/m]	[Hz]			
										Moas /Calo			
	Total Unight	20.5	Longth V [m]	25 F	Ciffeeers Datio	2	Number of	1	5000	Froguopov V	1.6		1 767224200
	Total Height	29.5	Length Y [m]	25.5	Sinness Ratio	2	Column Types	1	5000	fueluency, f	1.0		1.707334209
3										[[12]	L 🔺		∟
1	٨	D	C	D	F	F	C	ц			~		
	A	- D	L	D	E	F	G	п	1	J	N N	L	
	Number of E	Elements											
	Channel	Number of	Number of	Number									
1	Story	columns	shafts	Number of Walls							measur	od he	alculat
2	1	32	1	2							measure		arculat
:	2	32	1	8									
L	3	32	1	8									
5	4	32	1	8									
	5	32	1	8									
Τ		Stiffnesses	[MN/m]				Masses and	Weights					
-		Sumesses [Indusses and	weights	Clab dansity				
		Story	Kx	Ку			Story	Story Mass	Stab density	Story Height [m]	Slab Thickness	Weight [kN]	
				-			· ·	[kN/m]	[kN/m ²]		[m]	-	
		1	1224.574208	1867.018663			1	364.95	50	5.8	0.4	19961.415	
-		2	7560.66339	4861.827031			2	783	50	5.1	0.4	20816.1	
		3	7560.66339	4861.827031			3	783	50	5.1	0.4	20816.1	
		4	7560.66339	4861.827031			4	783	50	5.1	0.4	20385.45	
		5	9420.398476	6396.379231			5	783	50	4	0.4	18545.4	
L I		Calculated F	requencies [Hz]			Earthquake	Forces					
								-	Spectral				
		Mode	x	Y			lambda	1	acceleration	2.45		nput AUTF	1, GFZ
									Shear Force Fges				
		1	1.601816004	1.767334209			Weight [kN]	100524.465	[MN]	246.2849393			
		2	6.634602475	5.789989168	-				•				
		3	12.08460813	9.984985082									
		4	16.45251741	13.38894828									
		5	18.87130599	15.29247381									
,												Concrete Pro	nortios
-												Concreterre	perces
3	Column cros	ss-sections				Shaft cross-	sections					Strength [MPa]	30
	Current No.	X-Width [m]	Y-Width [m]	Radius [m]	Concrete cover	Current No.	X-Width [m]	Y-Width [m]	Wall Thickness,	Wall Thickness,		Reinforcement	0.015
					[m]				X [m]	Y [m]		ratio	
												Tons strongth	
	1	0.5	0.75	0	0.03	1	1.7	3.9	0.15	0.15		[MDa]	2.2
-	4 -											[ivira]	
	🖌 Dynai	mik										CIR	
• •	- í											-DD	
Ik				VD			finalmas	+ina 07	12 2016		20/27	0.0	10-
	l I			Y. Pe	euryna, It	JR, SIRAL	iinai mee	ung, U/	12.2010		29/3/		



Vulnerability assessment





Earthquake Forces					
lambda	1	Spectral acceleration	2.45 🔶		
Weight [kN]	100524.465	Shear Force Fges [MN]	246.2849393		

The design value of PGA is taken from the response spectrum according to AUTH investigations on-site.

235 Limit state assessment

236	X-direction				
237	Story	EQ Force [MN]	LS1 Force [MN]	LS2 Force [MN]	LS3 Force [MN]
238	1	246.2849393	4.746546254	20.78882127	22.87858781
239	2	195.7071645	31.89646838	301.1644601	1102.459691
240	3	141.3054095	27.6340501	290.0644197	1334.017535
241	4	88.76522569	23.37163172	279.5643801	1652.873636
242	5	40.65073695	30.49733346	441.6718571	3431.459118
240					









Case study: Technical school, L'Aquila, Italy



Partly damaged building unoccupied since 2009 RC frame, in-fill walls







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Main structural elements



Model check by use of vibration measurements

ilding												
lumber of Stories	5	Length X [m]	45.6	E-Modulus [MN/m²]	33000	Poisson's ratio	0.2	E-Modulus, Walls [MN/m²]	Meas./Calc. Frequency, X [Hz]	1.85		1.92180
otal Height	17.9	Length Y [m]	12.6	Siffness Ratio	2	Number of Column Types	1	4000	Meas./Calc. Frequency, Y [Hz]	1.65		1.68335
Number	of Elements									1		1
Story	Number o columns	f Number of shafts	Number of wa	alls						measur	red	calcula
1	39	0	3									
2	39	0	3									
3	39	0	3									
4 E	39	0	3	_								
<u> </u>	Stiffnesse	es [MN/m]	, ,									
	Story	Kx	Ку									
	1	1383.482236	5 1061.47155	7								
	2	1383.482236	5 1061.47155	7								
	3	1383.482236	5 1061.47155	7								
	4	1383.482236	5 1061.47155	7								
	5	1383.482236	5 1061.47155	7								
	Calculate	d Frequencie	s [Hz]	_		Masses	and Weights	;				
	Mode	×	Y	4		Stor	Story M y [kN/n	ass Slabde	nsity n ³ 1 Story Hei	ght [m] Slab	Thickness	Weight [kN]
	1	1.921804986	1.68335883	6		1	197.4	40	3.2	6	0.3	7479.024
		5 624507205	4.92665175	3		2	197.4	40	3.2	6	0.3	7479.024
	2	3.024307203					107.	1 40	2.7	c .	0.2	7479 024
	2	8.899745314	7.79551776	8		3	197.4	+ 40	3.2	.0	0.5	1475.024
	2 3 4	8.899745314 11.47545625	4 7.79551776 5 10.0516497	8 9		3	197.4	40 1 40	3.2	6	0.3	7479.024

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Vulnerability assessment at S_d = 0.2

Limit state assessment

X-direction

Story	EQ Force [MN]	LS1 Force [MN]	LS2 Force [MN]	LS3 Force [MN]
1	7.5929616	3.623893582	20.66379861	50.99124322
2	5.314472367	3.099581093	18.96430414	53.69999521
3	3.369922325	2.575268608	17.24186725	57.07878064
4	1.755473148	2.050956057	15.49226857	61.19380904
5	0.601242102	1.526643477	13.70921719	66.08850028

Y-direction				
Story	EQ Force [MN]	LS1 Force [MN]	LS2 Force [MN]	LS3 Force [MN]
1	7.5929616	3.509120301	19.48189042	41.47668039
2	5.314472367	3.001413451	17.70719499	42.9677038
3	3.369922325	2.493706603	15.91548907	44.94744365
4	1.755473148	1.985999692	14.10394802	47.46484746
5	0.601242102	1.478292752	12.2681631	50.55219537

Response Spectrum for L'Aquila

Record AM043 – Earthquake Spectra Response vs. NTC2008 Elastic Spectra for Civil Buildings

L. Petti, I. Marino (2009), Preliminary comparison between response spectra evaluated at close source for L'Aquila earthquake and elastic demand spectra according to new seismic Italian code (v.1.00), available at http://www.reluis.it

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Vulnerability assessment at S_d = 0.5 (record)

X-direction

Y-direction

 Earthquake Forces

 lambda
 1
 Spectral acceleration
 0.5

 Weight [kN]
 37964.808
 Shear Force Fges [MN]
 18.982404

Record acc. to Petti, Marino (2009)

Limit state assessment

x-direction				
Story	EQ Force [MN]	LS1 Force [MN]	LS2 Force [MN]	LS3 Force [MN]
1	18.982404	3.623893582	20.66379861	50.99124322
2	13.28618092	3.099581093	18.96430414	53.69999521
3	8.424805813	2.575268608	17.24186725	57.07878064
4	4.388682871	2.050956057	15.49226857	61.19380904
5	1.503105255	1.526643477	13.70921719	66.08850028

Y-direction				
Story	EQ Force [MN]	LS1 Force [MN]	LS2 Force [MN]	LS3 Force [MN]
1	18.982404	3.509120301	19.48189042	41.47668039
2	13.28618092	3.001413451	17.70719499	42.9677038
3	8.424805813	2.493706603	15.91548907	44.94744365
4	4.388682871	1.985999692	14.10394802	47.46484746
5	1.503105255	1.478292752	12.2681631	50.55219537

http://www.sibyl-project.eu/

Thanks for your attention

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